

R.3. IZRAČUN SIDRANE PILOTNE STENE

Analysis of anti-slide pile

Input data

Task : Rekonstrukcija ceste in usada na JP 965051 Lemberk-Vine v občini Vojnik
 Description : Sidrana pilotna stena
 Customer : GHC-Projekt, d.o.o.
 Author : Bojana Žibret
 Date : 1. 03. 2024
 Project ID : GG 49/3/24
 Project number : GG 49/3/24

Settings

Slovenia - EN 1997

Materials and standards

Concrete structures : EN 1992-1-1 (EC2)
 Coefficients EN 1992-1-1 : standard
 Steel structures : EN 1993-1-1 (EC3)
 Partial factor on bearing capacity of steel cross section : $\gamma_{M0} = 1,00$

Pressure analysis

Verification methodology : according to EN 1997
 Active earth pressure calculation : Coulomb
 Passive earth pressure calculation : Caquot-Kerisel
 Earthquake analysis : Mononobe-Okabe
 Modulus of subsoil reaction : standard
 Consider reduction of the modulus of subsoil reaction for a braced sheeting
 Pressures below the slip surface : standard
 Design approach : 2 - reduction of actions and resistances

Partial factors on actions (A)			
Permanent design situation			
		Unfavourable	Favourable
Permanent actions :	$\gamma_G =$	1,35 [-]	1,00 [-]
Variable actions :	$\gamma_Q =$	1,50 [-]	0,00 [-]
Water load :	$\gamma_w =$	1,35 [-]	

Partial factors for resistances (R)			
Permanent design situation			
Partial factor on earth resistance :		$\gamma_{Re} =$	1,40 [-]

Anchors

Verification methodology : Limit states (LSD)

Reduction coefficients			
Reduction. coeff of steel strength :		$\gamma_s =$	1,35 [-]
Reduction coefficient of pull out resistance (soil) :		$\gamma_e =$	1,35 [-]
Reduction coefficient of pull out resistance (grouting) :		$\gamma_c =$	1,35 [-]

Geometry of structure

Structure length = 14,50 m

Cross-section name : Pile curtain d = 0.50 m, a = 1.00 m
 Material of pile : concrete
 Computed coefficient of pressure reduction below the ditch = 1,00
 Area of cross-section $A = 1,96E-01 \text{ m}^2/\text{m}$

Moment of inertia $I = 3,07E-03 \text{ m}^4/\text{m}$

Forces above the slip surface

Depth of slip surface $h_{s1} = 2,50 \text{ m}$

Input of active horizontal force : residual active force

Input of passive horizontal force : residual passive force

Active horizontal force $T = 0,00 \text{ kN/m}$

Passive horizontal force $P = 0,00 \text{ kN/m}$

Distribution of active force : triangle

Distribution of passive force : triangle

Material of structure

Analysis of concrete structures carried out according to the standard EN 1992-1-1 (EC2).

Concrete: C 20/25

Cylinder compressive strength $f_{ck} = 20,00 \text{ MPa}$

Tensile strength $f_{ctm} = 2,20 \text{ MPa}$

Elasticity modulus $E_{cm} = 30000,00 \text{ MPa}$

Shear modulus $G = 12500,00 \text{ MPa}$

Longitudinal steel: B500B

Yield strength $f_{yk} = 500,00 \text{ MPa}$

Transverse steel: B500B

Yield strength $f_{yk} = 500,00 \text{ MPa}$

Modulus of reaction

Modulus of subsoil reaction is computed by method Schmitt.

Basic soil parameters

No.	Name	Pattern	Φ_{ef} [°]	C_{ef} [kPa]	γ [kN/m ³]	γ_{su} [kN/m ³]	δ [°]
1	Gruščnata peščena glina		34,00	4,00	18,00	9,00	0,00
2	Lapor, apnenec		35,00	100,00	22,00	12,00	0,00
3	Tampon		35,00	0,00	20,00	11,00	0,00

Soil parameters to compute pressure at rest

No.	Name	Pattern	Type calculation	Φ_{ef} [°]	ν [-]	OCR [-]	K_r [-]
1	Gruščnata peščena glina		cohesive	-	0,40	-	-
2	Lapor, apnenec		cohesive	-	0,30	-	-
3	Tampon		cohesive	-	0,30	-	-

Parameters of soils to compute modulus of subsoil reaction (Schmitt)

No.	Name	Pattern	ν [-]	E_{oed} [MPa]	E_{def} [MPa]
1	Gruščnata peščena glina		0,40	-	5,00
2	Lapor, apnenec		0,30	-	100,00

No.	Name	Pattern	ν [-]	E_{oed} [MPa]	E_{def} [MPa]
3	Tampon		0,30	-	100,00

Soil parameters

Grušcnata peščena glina

Unit weight : $\gamma = 18,00 \text{ kN/m}^3$
 Stress-state : effective
 Angle of internal friction : $\varphi_{ef} = 34,00^\circ$
 Cohesion of soil : $c_{ef} = 4,00 \text{ kPa}$
 Angle of friction struc.-soil : $\delta = 0,00^\circ$
 Soil : cohesive
 Poisson's ratio : $\nu = 0,40$
 Deformation modulus : $E_{def} = 5,00 \text{ MPa}$
 Poisson's ratio : $\nu = 0,40$
 Saturated unit weight : $\gamma_{sat} = 19,00 \text{ kN/m}^3$

Lapor, apnenec

Unit weight : $\gamma = 22,00 \text{ kN/m}^3$
 Stress-state : effective
 Angle of internal friction : $\varphi_{ef} = 35,00^\circ$
 Cohesion of soil : $c_{ef} = 100,00 \text{ kPa}$
 Angle of friction struc.-soil : $\delta = 0,00^\circ$
 Soil : cohesive
 Poisson's ratio : $\nu = 0,30$
 Deformation modulus : $E_{def} = 100,00 \text{ MPa}$
 Poisson's ratio : $\nu = 0,30$
 Saturated unit weight : $\gamma_{sat} = 22,00 \text{ kN/m}^3$

Tampon

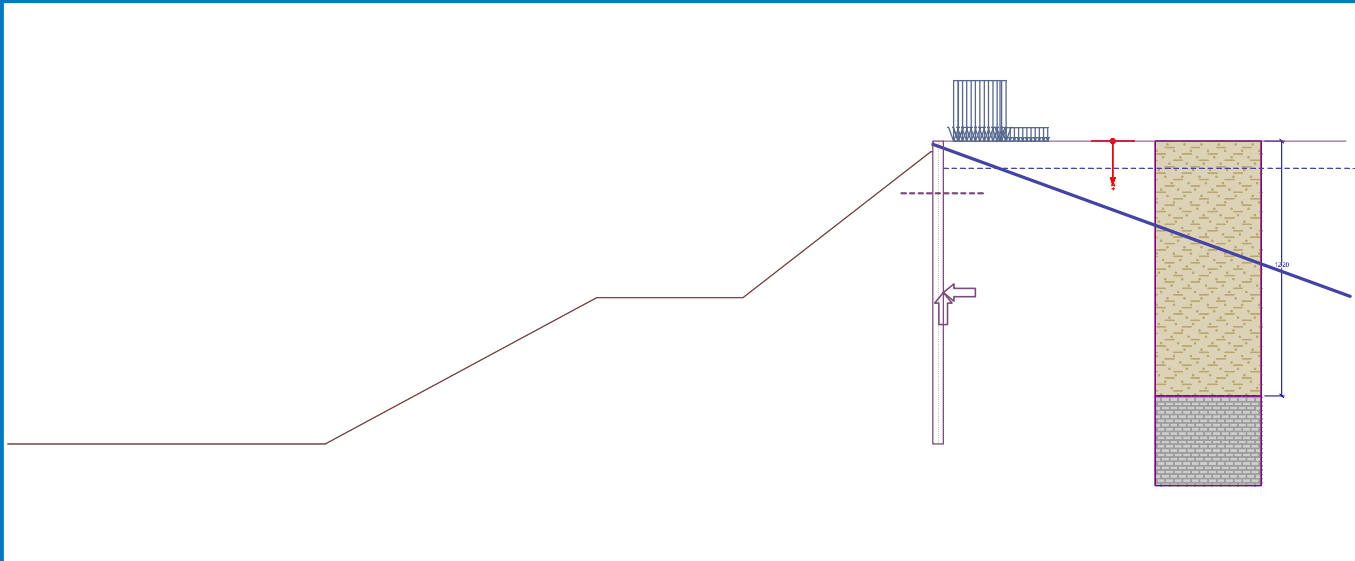
Unit weight : $\gamma = 20,00 \text{ kN/m}^3$
 Stress-state : effective
 Angle of internal friction : $\varphi_{ef} = 35,00^\circ$
 Cohesion of soil : $c_{ef} = 0,00 \text{ kPa}$
 Angle of friction struc.-soil : $\delta = 0,00^\circ$
 Soil : cohesive
 Poisson's ratio : $\nu = 0,30$
 Deformation modulus : $E_{def} = 100,00 \text{ MPa}$
 Poisson's ratio : $\nu = 0,30$
 Saturated unit weight : $\gamma_{sat} = 21,00 \text{ kN/m}^3$

Geological profile and assigned soils

No.	Thickness of layer t [m]	Depth z [m]	Assigned soil	Pattern
1	12,20	0,00 .. 12,20	Grušcnata peščena glina	
2	-	12,20 .. ∞	Lapor, apnenec	

Name : Profile and assignment

Stage - analysis : 1 - 0



Excavation

Soil in front of wall is excavated to a depth of 0,50 m.

Ditch bottom shape

No.	Coordinate x [m]	Depth z [m]
1	0,00	0,00
2	-0,10	0,00
3	-9,10	7,00
4	-16,10	7,00
5	-29,10	14,00
6	-30,10	14,00

Origin [0,0] is located at the ditch bottom.

Positive coordinate +z has downward direction.

Terrain profile

Terrain behind the structure is flat.

Water influence

GWT behind the structure lies at a depth of 1,30 m

Input surface surcharges

No.	Surcharge		Action	Mag.1 [kN/m ²]	Mag.2 [kN/m ²]	Ord.x x [m]	Length l [m]	Depth z [m]
	new	change						
1	Yes		variable	9,00		0,50	2,50	on terrain
2	Yes		variable	2,00		3,00	2,00	on terrain

Input concentrated surcharges

No.	Surcharge		Action	Magnitude [kN]	Ord.x x [m]	Length l [m]	Width b[m]	Depth z [m]
	new	change						
1	Yes		permanent	65,00	0,50	0,44	0,32	on terrain
2	Yes		permanent	65,00	2,50	0,44	0,32	on terrain

Input anchors

No.	New anchor	Depth z [m]	Name	Post-stressing	Force F [kN]
1	Yes	0,25	IBO SIS-R38-500 (user-defined)		399,16

List of the new anchors**IBO SIS-R38-500 (user-defined)**

Anchor type : non-prestressed bar

Production set : user-defined

Depth : $z = 0,25 \text{ m}$ Overall length : $l = 21,00 \text{ m}$ Slope : $\alpha = 20,00^\circ$ Spacing : $b = 4,00 \text{ m}$ Diameter : $d_s = 38,00 \text{ mm}$ Elasticity modulus : $E = 210000,00 \text{ MPa}$ Design strength of material : $f_u = 690,00 \text{ MPa}$ Pull out resistance (soil) : $R_e = 15187,75 \text{ kN}$ **Earthquake**Factor of horizontal acceleration $K_h = 0,0350$ Factor of vertical acceleration $K_v = 0,0350$

Water below the GWT is restricted.

Global settings

Number of FEs to discretize wall = 100

Analysis of depending pressures : reduce according to analysis settings

Minimum pressure is considered as $\sigma_{a,min} = 0,20\sigma_z$ **Settings of the stage of construction**

Design situation : permanent

Analysis results**Pressure above the slip surface**

Depth [m]	Passive pressure [kPa]	Active pressure [kPa]
0	0,00	0,00
0,50	0,00	0,00
0,50	0,00	0,00
2,50	0,00	0,00

Distribution of pressures acting on the structure (in front and behind the wall) - below the slip surface

Depth [m]	Ta,p [kPa]	Tk,p [kPa]	Tp,p [kPa]	Ta,z [kPa]	Tk,z [kPa]	Tp,z [kPa]
2.50	-5.54	-16.40	-22.74	31.31	34.80	107.80
2.51	-8.18	-16.43	-22.79	36.21	39.30	96.33
2.64	-8.75	-17.47	-23.93	38.16	41.05	100.44
3.30	-11.59	-22.67	-29.65	47.97	50.25	121.08
3.95	-14.42	-27.87	-35.37	57.77	59.95	141.71
4.61	-17.26	-33.07	-41.09	67.58	69.97	162.34
5.12	-19.42	-37.03	-45.45	75.04	77.72	178.06
5.27	-20.10	-38.27	-46.81	77.60	80.15	182.98
5.64	-21.69	-41.18	-50.02	83.62	85.92	194.54
5.64	-21.69	-41.18	-50.02	80.80	85.92	194.54
5.93	-22.94	-43.47	-52.54	85.57	90.44	203.61
6.59	-25.78	-48.66	-58.26	96.44	100.81	224.24
7.00	-27.54	-51.89	-61.81	103.18	107.28	237.05
7.00	-27.54	-51.90	-61.82	103.20	107.29	237.08
7.25	-28.61	-53.86	-63.98	107.30	111.23	244.88
7.50	-29.69	-55.84	-66.15	111.43	115.20	252.70

Depth [m]	Ta,p [kPa]	Tk,p [kPa]	Tp,p [kPa]	Ta,z [kPa]	Tk,z [kPa]	Tp,z [kPa]
7.91	-31.45	-59.06	-69.70	118.17	121.68	265.51
8.57	-34.29	-64.26	-75.43	129.04	132.16	286.14
8.96	-35.98	-67.36	-78.84	135.52	138.42	298.46
9.23	-37.13	-69.46	-91.19	139.90	142.65	306.78
9.40	-37.89	-70.85	-99.40	142.81	145.46	312.30
9.40	-37.89	-70.85	-99.40	142.64	145.46	312.30
9.89	-39.97	-74.66	-121.85	150.63	153.16	327.41
10.55	-42.80	-79.86	-152.50	161.54	163.67	348.04
11.20	-45.64	-85.06	-183.15	172.46	174.19	368.68
11.86	-48.48	-90.25	-213.80	183.37	184.72	389.31
12.20	-49.93	-92.91	-229.44	188.94	190.09	399.84
12.20	-0.58	-61.74	-430.06	148.13	161.16	670.34
12.52	-0.50	-63.81	-437.61	152.35	166.04	683.21
13.18	-0.33	-68.03	-453.02	160.96	176.01	709.49
13.84	-0.17	-72.25	-468.43	169.58	185.98	735.76
14.00	-0.13	-73.27	-472.15	171.66	188.39	742.10
14.50	0.00	-76.48	-483.84	178.20	195.96	762.04

Distributions of the modulus of subsoil reaction and internal forces on the structure

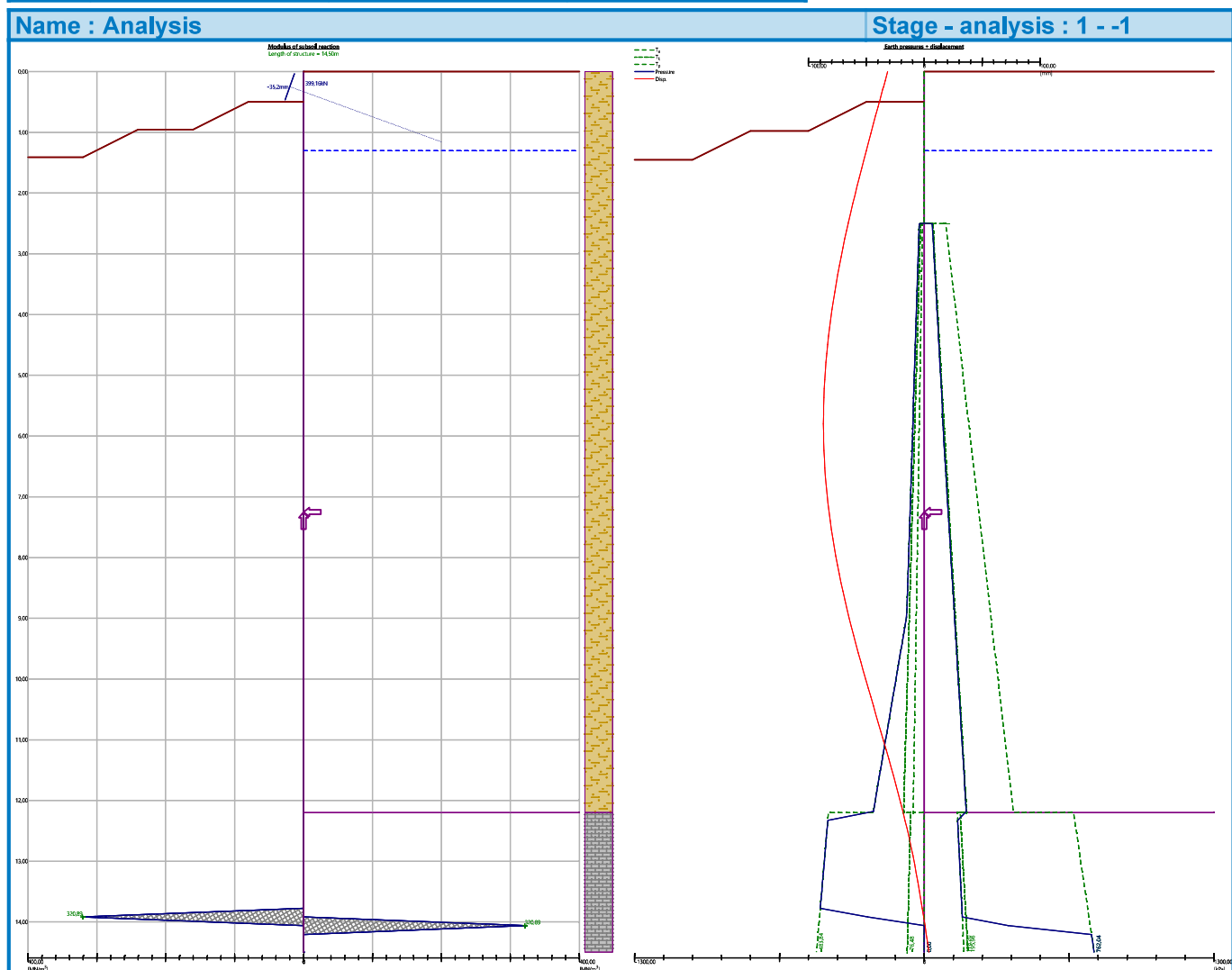
Depth [m]	kh,p [MN/m ³]	kh,z [MN/m ³]	Displacement [mm]	Pressure [kPa]	Shear Force [kN/m]	Moment [kNm/m]
0.00	0.00	0.00	-31.59	0.00	-0.00	-0.00
0.25	0.00	0.00	-35.20	0.00	-0.00	0.00
0.25	0.00	0.00	-35.20	0.00	93.77	0.00
0.50	0.00	0.00	-38.74	0.00	93.77	-23.07
0.50	0.00	0.00	-38.85	0.00	93.77	-23.82
0.73	0.00	0.00	-42.02	0.00	93.77	-44.54
1.45	0.00	0.00	-52.19	0.00	93.77	-112.53
2.17	0.00	0.00	-61.71	0.00	93.77	-180.51
2.50	0.00	0.00	-65.63	0.00	93.77	-210.61
2.50	0.00	0.00	-65.72	13.42	93.72	-211.36
2.90	0.00	0.00	-70.21	15.87	87.92	-247.36
3.63	0.00	0.00	-77.30	20.36	74.79	-306.54
4.35	0.00	0.00	-82.64	24.85	58.40	-355.01
5.08	0.00	0.00	-85.96	29.34	38.75	-390.42
5.80	0.00	0.00	-87.05	32.01	16.13	-410.41
6.53	0.00	0.00	-85.82	37.66	-9.13	-413.19
7.25	0.00	0.00	-82.23	43.32	-38.49	-396.18
7.98	0.00	0.00	-76.39	48.98	-71.95	-356.40
8.70	0.00	0.00	-68.52	54.64	-109.51	-290.87
9.43	0.00	0.00	-59.01	42.60	-147.04	-197.28
10.15	0.00	0.00	-48.39	20.89	-170.05	-81.38
10.88	0.00	0.00	-37.30	-0.82	-177.33	45.50
11.60	0.00	0.00	-26.47	-22.53	-168.86	171.95
12.33	0.00	0.00	-16.62	-283.22	-127.33	285.71
13.05	0.00	0.00	-8.35	-290.70	80.72	302.93
13.78	0.00	0.00	-1.74	-298.17	294.18	167.36

Depth [m]	kh,p [MN/m ³]	kh,z [MN/m ³]	Displacement [mm]	Pressure [kPa]	Shear Force [kN/m]	Moment [kNm/m]
14.50	0.00	0.00	3.96	762.04	-0.00	0.00

Maximum shear force = 335,48 kN/m
 Maximum moment = 414,29 kNm/m
 Maximum displacement = 87,1 mm
 Displacement in the depth of slip surface = 65,7 mm

Anchors forces

No.	Depth [m]	Displacement [mm]	Anchor force [kN]
1	0,25	-35,2	399,16



Dimensioning No. 1

Distribution of forces on construction

	Disp. min [mm]	Disp. max [mm]	Shear force min. [kN/m]	Shear force max [kN/m]	Moment min. [kNm/m]	Moment max. [kNm/m]
0.00	-31.59	-31.59	-0.00	-0.00	-0.00	-0.00
0.25	-35.20	-35.20	-0.00	-0.00	0.00	0.00
0.25	-35.20	-35.20	93.77	93.77	0.00	0.00

	Disp. min [mm]	Disp. max [mm]	Shear force min. [kN/m]	Shear force max [kN/m]	Moment min. [kNm/m]	Moment max. [kNm/m]
0.50	-38.74	-38.74	93.77	93.77	-23.07	-23.07
0.50	-38.74	-38.74	93.77	93.77	-23.07	-23.07
0.50	-38.85	-38.85	93.77	93.77	-23.82	-23.82
0.50	-38.85	-38.85	93.77	93.77	-23.82	-23.82
0.73	-42.02	-42.02	93.77	93.77	-44.54	-44.54
1.45	-52.19	-52.19	93.77	93.77	-112.53	-112.53
2.17	-61.71	-61.71	93.77	93.77	-180.51	-180.51
2.50	-65.63	-65.63	93.77	93.77	-210.61	-210.61
2.50	-65.63	-65.63	93.77	93.77	-210.61	-210.61
2.50	-65.72	-65.72	93.72	93.72	-211.36	-211.36
2.50	-65.72	-65.72	93.72	93.72	-211.36	-211.36
2.90	-70.21	-70.21	87.92	87.92	-247.36	-247.36
3.63	-77.30	-77.30	74.79	74.79	-306.54	-306.54
4.35	-82.64	-82.64	58.40	58.40	-355.01	-355.01
5.08	-85.96	-85.96	38.75	38.75	-390.42	-390.42
5.80	-87.05	-87.05	16.13	16.13	-410.41	-410.41
6.53	-85.82	-85.82	-9.13	-9.13	-413.19	-413.19
7.25	-82.23	-82.23	-38.49	-38.49	-396.18	-396.18
7.98	-76.39	-76.39	-71.95	-71.95	-356.40	-356.40
8.70	-68.52	-68.52	-109.51	-109.51	-290.87	-290.87
9.43	-59.01	-59.01	-147.04	-147.04	-197.28	-197.28
10.15	-48.39	-48.39	-170.05	-170.05	-81.38	-81.38
10.88	-37.30	-37.30	-177.33	-177.33	45.50	45.50
11.60	-26.47	-26.47	-168.86	-168.86	171.95	171.95
12.33	-16.62	-16.62	-127.33	-127.33	285.71	285.71
13.05	-8.35	-8.35	80.72	80.72	302.93	302.93
13.78	-1.74	-1.74	294.18	294.18	167.36	167.36
14.50	3.96	3.96	-0.00	-0.00	0.00	0.00

Maximum values of internal forces

Maximum displacement = -87,1 mm
 Minimum displacement = 4,0 mm
 Maximum bending moment = 314,16 kNm/m
 Minimum bending moment = -414,29 kNm/m
 Maximum shear force = 335,48 kN/m

Verification of RC cross section (Pile curtain d = 0.50 m, a = 1.00 m)

All construction stages are taken into the analysis.

Partial factor on load = 1,00

Verification of cross section in bending:

Reinforcement - 12 pc bars 32,0 mm; cover 80,0 mm

Type of structure (reinforcement ratio) : beam

Reinforcement ratio $\rho = 2,458 \% > 0,130 \% = \rho_{\min}$

Load : $M_{Ed} = 414,29 \text{ kNm}$

Bearing capacity : $M_{Rd} = 434,85 \text{ kNm}$

Designed pile reinforcement is SATISFACTORY

Verification of cross section in shear:

Shear reinf. - profile 10,0 mm; spacing 150,0 mm

$A_{sw} = 2 \times 523,6 = 1047,2 \text{ mm}^2$

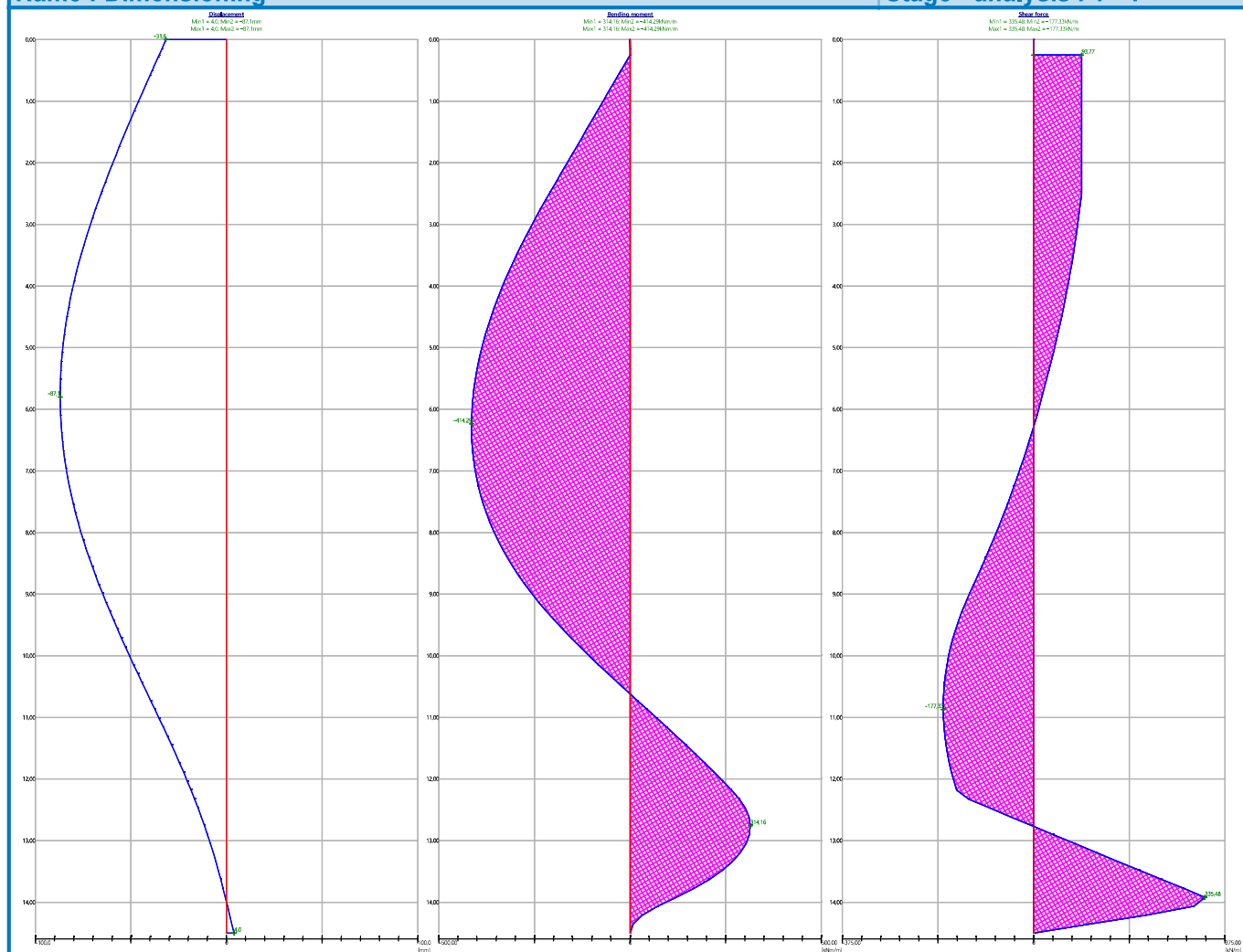
Ultimate shear force: $V_{Rd} = 409,77 \text{ kN} > 335,48 \text{ kN} = V_{Ed}$

Cross-section is SATISFACTORY.

Overall verification: Cross-section is SATISFACTORY

Name : Dimensioning

Stage - analysis : 1 - 1



Verification of lagging 1

Input data

Concrete : C 30/37

Longitudinal steel : B500B

Transverse steel : B500B

Cross section type : rectangleb x h = 1200,0 x 500,0 mm

Type of load : rectangle

Verification of RC cross section (b = 0,50 m; h = 1,20 m)

All construction stages are taken into the analysis.

Partial factor on load = 1,00

Reinforcement - 10 pc bars 12,0 mm; cover 50,0 mm

Shear reinf. - 2 profile 10,0 mm; distance 200,0 mm

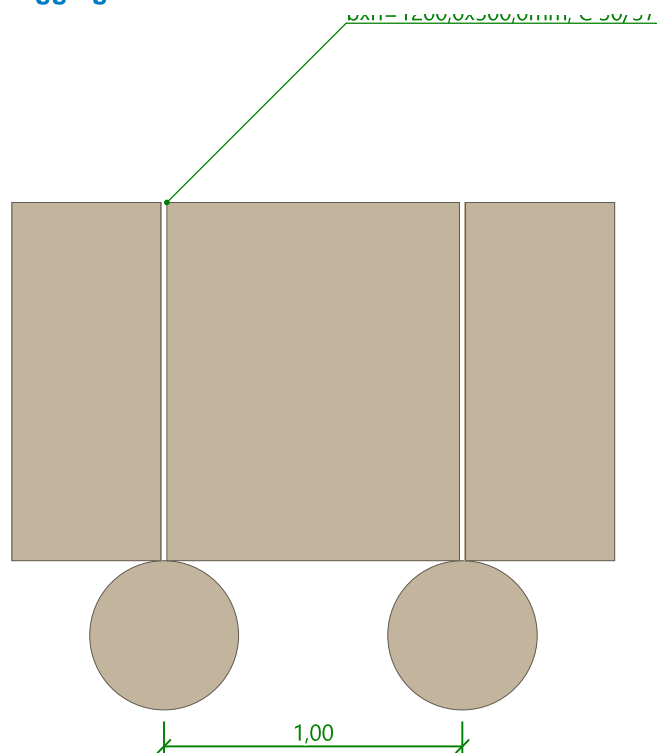
Reinforcement ratio $\rho = 0,20 \% > 0,15 \% = \rho_{min}$

Position of neutral axis $x = 0,06 \text{ m} < 0,71 \text{ m} = x_{max}$

Ultimate moment $M_{Rd} = 550,45 \text{ kNm} > 0,00 \text{ kNm} = M_{Ed}$

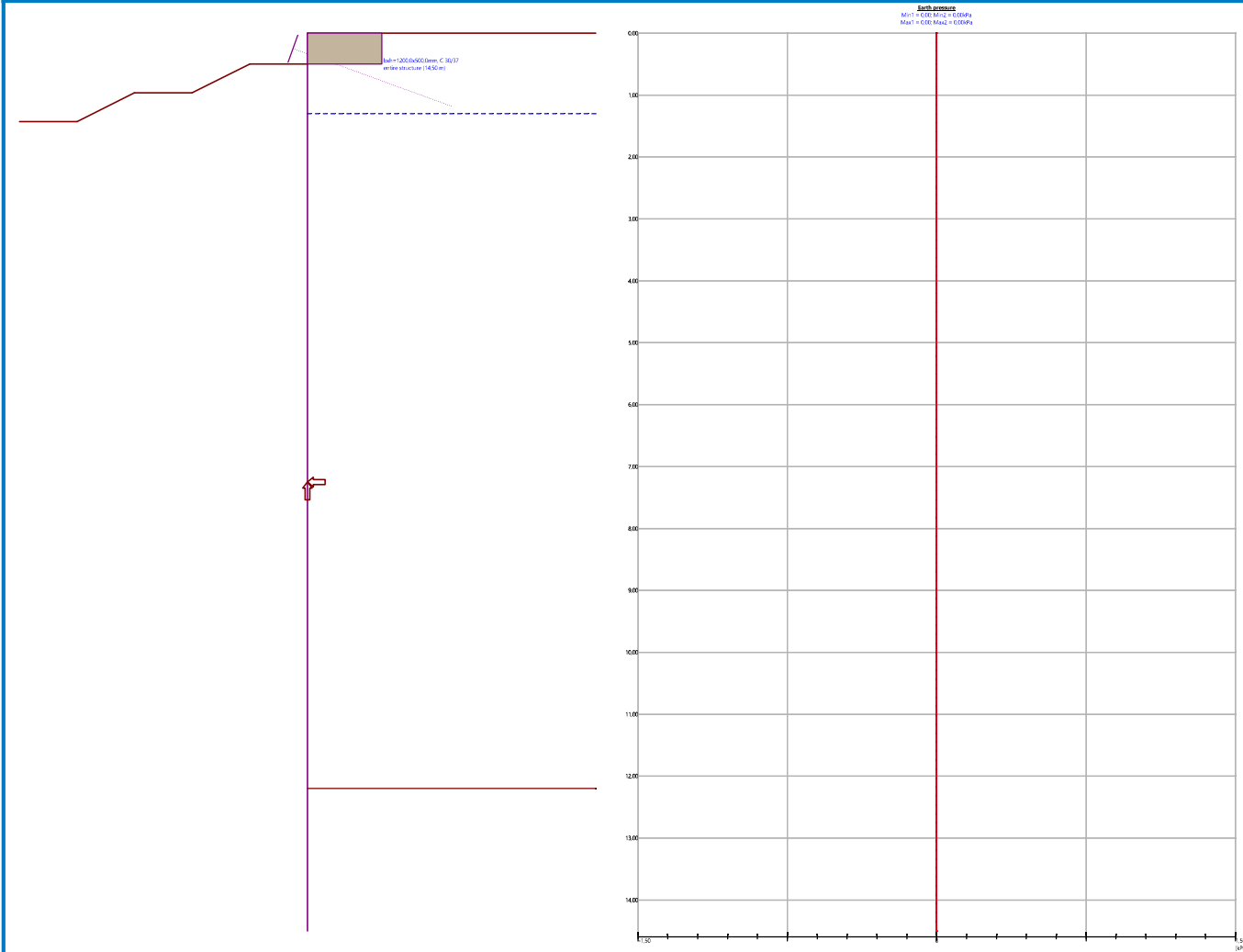
Cross-section is SATISFACTORY.

Lagging scheme



Name : Lagging

Stage - analysis : 1 - 1



Verification of anchors

Anchor	Stage	Depth z [m]	Max. force F [kN]	Anchor strength R _t [kN]	Pull-out res. (soil) R _e [kN]	Pull-out res. (grouting) R _c [kN]	Verification
1	1	0,25	399,16	579,66	11250,19	-	is satisfactory (68,86 %)

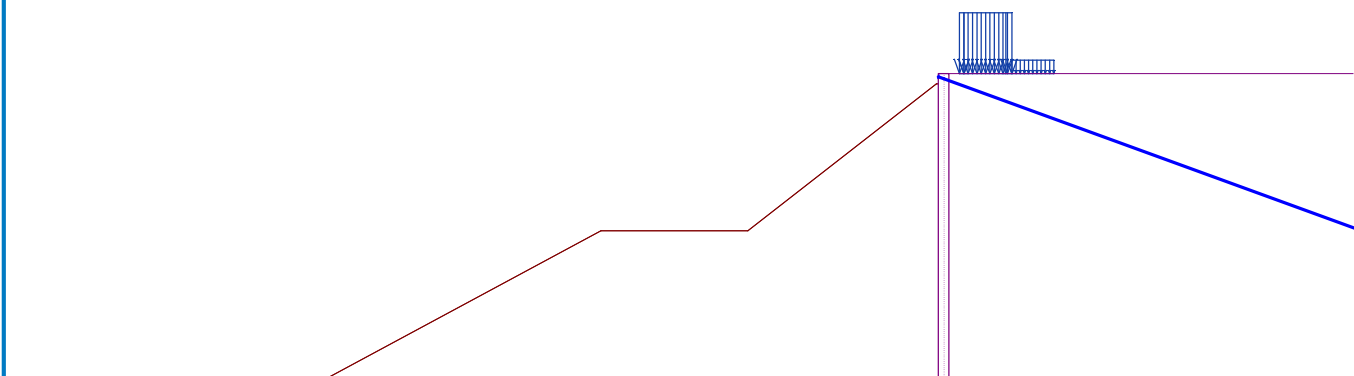
Anchor with max. utilization - Nr. 1. (Stage 1; z = 0,25 m)

Utilization is 68,86 %

Anchors bearing capacity is SATISFACTORY

Name : Anchors Verification

Stage - analysis : 1 - -1



R.4. ANALIZA STABILNOSTI PILOTNE STENE PO SANACIJI

Slope stability analysis

Input data

Project

Task : Rekonstrukcija ceste in usada na JP 965051 Lemberg-Vine v občini Vojnik
 Part : Analiza stabilnosti po sanaciji
 Description : Sidrana pilotna stena
 Customer : GHC-Projekt, d.o.o.
 Author : Ciril Obreza, dipl. inž. grad.
 Date : 1. 03. 2024
 Project ID : GG 49/3/24
 Project number : GG 49/3/24

Settings

Slovenia - EN 1997

Stability analysis

Earthquake analysis : Standard
 Verification methodology : according to EN 1997
 Design approach : 3 - reduction of actions (GEO, STR) and soil parameters

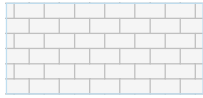
Partial factors on actions (A)					
Permanent design situation					
		State STR		State GEO	
		Unfavourable	Favourable	Unfavourable	Favourable
Permanent actions :	$\gamma_G =$	1,00 [-]	1,00 [-]	1,00 [-]	1,00 [-]
Variable actions :	$\gamma_Q =$	1,50 [-]	0,00 [-]	1,30 [-]	0,00 [-]
Water load :	$\gamma_w =$			1,00 [-]	

Partial factors for soil parameters (M)		
Permanent design situation		
Partial factor on internal friction :	$\gamma_\phi =$	1,25 [-]
Partial factor on effective cohesion :	$\gamma_c =$	1,25 [-]
Partial factor on undrained shear strength :	$\gamma_{cu} =$	1,40 [-]

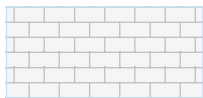
Interface

No.	Interface location	Coordinates of interface points [m]					
		x	z	x	z	x	z
1		-7,40	-0,99	-7,10	-0,90	5,99	2,91
		17,86	10,00	28,05	11,53	33,40	14,10
		37,15	18,16	43,74	18,16		
2		-7,40	-3,16	37,15	5,96	43,74	16,83

Soil parameters - effective stress state

No.	Name	Pattern	φ_{ef} [°]	c_{ef} [kPa]	γ [kN/m ³]
1	Grušcnata peščenca glina		34,00	4,00	18,00
2	Lapor, apnenec		35,00	100,00	22,00

Soil parameters - uplift

No.	Name	Pattern	γ_{sat} [kN/m ³]	γ_s [kN/m ³]	n [-]
1	Grušcnata peščenca glina		19,00		
2	Lapor, apnenec		22,00		

Soil parameters

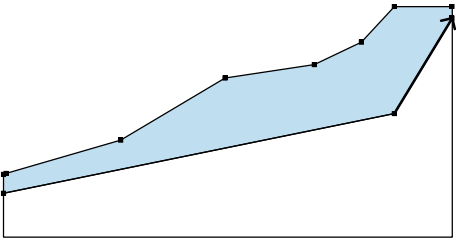
Grušcnata peščenca glina

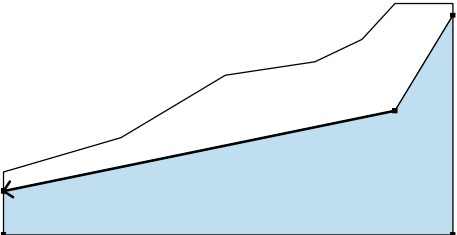
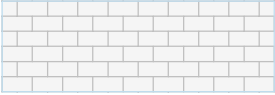
Unit weight : $\gamma = 18,00 \text{ kN/m}^3$
 Stress-state : effective
 Angle of internal friction : $\varphi_{ef} = 34,00^\circ$
 Cohesion of soil : $c_{ef} = 4,00 \text{ kPa}$
 Saturated unit weight : $\gamma_{sat} = 19,00 \text{ kN/m}^3$

Lapor, apnenec

Unit weight : $\gamma = 22,00 \text{ kN/m}^3$
 Stress-state : effective
 Angle of internal friction : $\varphi_{ef} = 35,00^\circ$
 Cohesion of soil : $c_{ef} = 100,00 \text{ kPa}$
 Saturated unit weight : $\gamma_{sat} = 22,00 \text{ kN/m}^3$

Assigning and surfaces

No.	Surface position	Coordinates of surface points [m]				Assigned soil
		x	z	x	z	
1		37,15	5,96	43,74	16,83	Grušcnata peščenca glina 
		43,74	18,16	37,15	18,16	
		33,40	14,10	28,05	11,53	
		17,86	10,00	5,99	2,91	
		-7,10	-0,90	-7,40	-0,99	
		-7,40	-3,16			

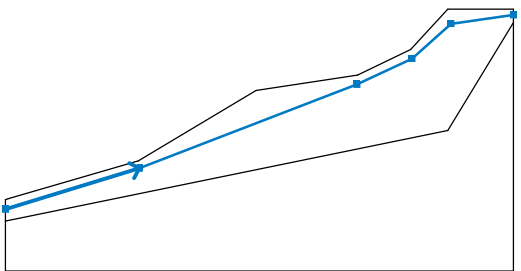
No.	Surface position	Coordinates of surface points [m]				Assigned soil
		x	z	x	z	
2		37,15	5,96	-7,40	-3,16	Lapor, apnenec 
		-7,40	-8,16	43,74	-8,16	
		43,74	16,83			

Anti-Slide piles

No.	Point		Length l [m]	Pile spacing b [m]	Cross-section [m]	Distribution along the pile	Pile bearing capacity		
	x [m]	z [m]					Max. bearing capacity V _u [kN]	Gradient K [-]	Passive force direction
1	37,21	18,16	14,50	1,00	d = 0,50	linear	409,77	1,00	perpendicular to pile

Water

Water type : GWT

No.	GWT location	Coordinates of GWT points [m]					
		x	z	x	z	x	z
1		-7,40	-1,95	6,11	2,16	27,99	10,61
		33,54	13,19	37,45	16,70	43,74	17,60

Tensile crack

Tensile crack not input.

Earthquake

Earthquake not included.

Settings of the stage of construction

Design situation : permanent

Results (Stage of construction 1)**Analysis 1 (stage 1)****Circular slip surface**

Slip surface parameters						
Center :	x =	0,53 [m]	Angles :	α ₁ =	-5,99 [°]	
	z =	50,08 [m]		α ₂ =	50,86 [°]	
Radius :	R =	50,57 [m]				
The slip surface after optimization.						

The restrictions of points of circular slip surface

Keep the right end point of the slip surface

Slope stability verification (Bishop)Sum of active forces : F_a = 1164,74 kN/m

Sum of passive forces : $F_p = 1562,69 \text{ kN/m}$

Sliding moment : $M_a = 58901,07 \text{ kNm/m}$

Resisting moment : $M_p = 79025,05 \text{ kNm/m}$

Utilization : 74,5 %

Slope stability ACCEPTABLE

Piles verification 1 (stage 1)

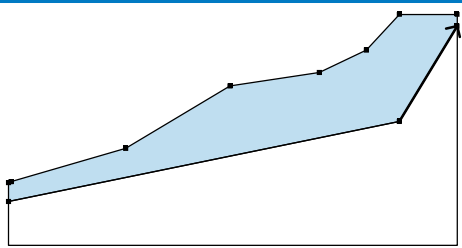
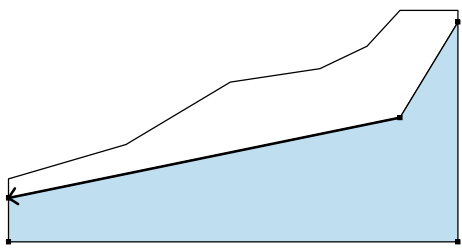
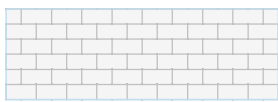
Anti-Slide pile : Anti-Slide Pile No. 1 (37,21; 18,16 [m])

Analysis : Calculation 1 (slip surface circular)

Method : Bishop

Input data (Stage of construction 2)

Assigning and surfaces

No.	Surface position	Coordinates of surface points [m]				Assigned soil
		x	z	x	z	
1		37,15	5,96	43,74	16,83	Gruščnata peščena glina 
		43,74	18,16	37,15	18,16	
		33,40	14,10	28,05	11,53	
		17,86	10,00	5,99	2,91	
		-7,10	-0,90	-7,40	-0,99	
		-7,40	-3,16			
2		37,15	5,96	-7,40	-3,16	Lapor, apnenec 
		-7,40	-8,16	43,74	-8,16	
		43,74	16,83			

Anti-Slide piles

No.	Anti-Slide pile	Point		Length	Pile spacing	Cross-section	Pile bearing capacity			
	new	x [m]	z [m]	l [m]	b [m]	[m]	Distribution along the pile	Max. bearing capacity V_u [kN]	Gradient K [-]	Passive force direction
1	No	37,21	18,16	14,50	1,00	d = 0,50	linear	409,77	1,00	perpendicular to pile

Water

Water type : No water

Tensile crack

Tensile crack not input.

Earthquake

Horizontal seismic coefficient : $K_h = 0,04$

Vertical seismic coefficient : $K_v = 0,04$

Settings of the stage of construction

Design situation : permanent

Results (Stage of construction 2)

Analysis 1 (stage 2)

Circular slip surface

Slip surface parameters					
Center :	x =	2,95 [m]	Angles :	α_1 =	-7,05 [°]
	z =	47,90 [m]		α_2 =	51,67 [°]
Radius :	R =	47,95 [m]			
The slip surface after optimization.					

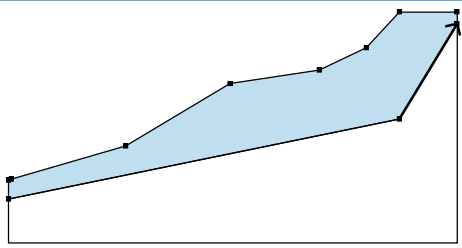
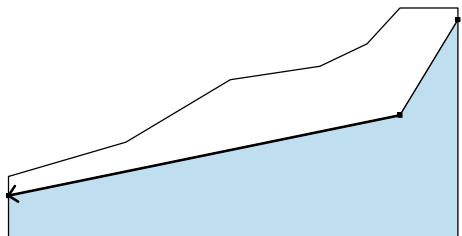
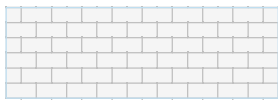
The restrictions of points of circular slip surface

Keep the right end point of the slip surface

Slope stability verification (Bishop)Sum of active forces : $F_a = 1252,29 \text{ kN/m}$ Sum of passive forces : $F_p = 2032,36 \text{ kN/m}$ Sliding moment : $M_a = 60047,23 \text{ kNm/m}$ Resisting moment : $M_p = 97451,67 \text{ kNm/m}$

Utilization : 61,6 %

Slope stability ACCEPTABLE**Input data (Stage of construction 3)****Assigning and surfaces**

No.	Surface position	Coordinates of surface points [m]				Assigned soil
		x	z	x	z	
1		37,15	5,96	43,74	16,83	Gruščnata peščena glina 
		43,74	18,16	37,15	18,16	
		33,40	14,10	28,05	11,53	
		17,86	10,00	5,99	2,91	
		-7,10	-0,90	-7,40	-0,99	
		-7,40	-3,16			
2		37,15	5,96	-7,40	-3,16	Lapor, apnenec 
		-7,40	-8,16	43,74	-8,16	
		43,74	16,83			

Anti-Slide piles

No.	Anti-Slide pile	Point		Length	Pile spacing	Cross-section	Pile bearing capacity			
	new	x [m]	z [m]	l [m]	b [m]	[m]	Distribution along the pile	Max. bearing capacity V_u [kN]	Gradient K [-]	Passive force direction
1	No	37,21	18,16	14,50	1,00	d = 0,50	linear	409,77	1,00	perpendicular to pile

Surcharge

No.	Surcharge		Type	Type of action	Location	Origin	Length	Width	Slope	Magnitude		
	new	change			z [m]	x [m]	l [m]	b [m]	α [°]	q, q_1, f, F	q_2	unit
1	Yes		strip	variable	on terrain	$x = 38,00$	$l = 2,50$		0,00	9,00		kN/m ²
2	Yes		strip	variable	on terrain	$x = 40,50$	$l = 2,00$		0,00	2,00		kN/m ²

No.	Surcharge		Type	Type of action	Location z [m]	Origin x [m]	Length l [m]	Width b [m]	Slope α [°]	Magnitude		
	new	change								q, q ₁ , f, F	q ₂	unit
3	Yes		concentrated	variable	on terrain	x = 38,00	l = 0,44	b = 0,32		65,00		kN
4	Yes		concentrated	variable	on terrain	x = 40,50	l = 0,44	b = 0,32		65,00		kN

Water

Water type : No water

Tensile crack

Tensile crack not input.

Earthquake

Earthquake not included.

Settings of the stage of construction

Design situation : permanent

Results (Stage of construction 3)**Analysis 1 (stage 3)****Circular slip surface**

Slip surface parameters					
Center :	x =	28,74 [m]	Angles :	α_1 =	-18,33 [°]
	z =	20,78 [m]		α_2 =	75,03 [°]
Radius :	R =	10,14 [m]			
The slip surface after optimization.					

The restrictions of points of circular slip surface

Keep the right end point of the slip surface

Slope stability verification (Bishop)Sum of active forces : $F_a = 289,47$ kN/mSum of passive forces : $F_p = 559,48$ kN/mSliding moment : $M_a = 2935,24$ kNm/mResisting moment : $M_p = 5673,15$ kNm/m

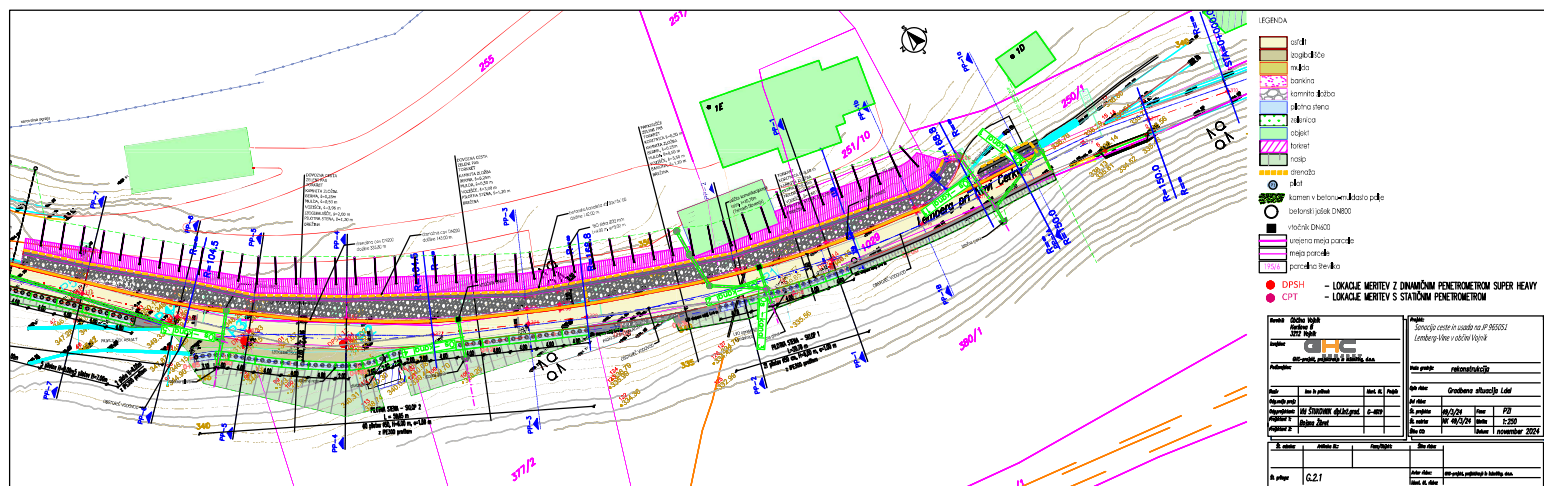
Utilization : 51,7 %

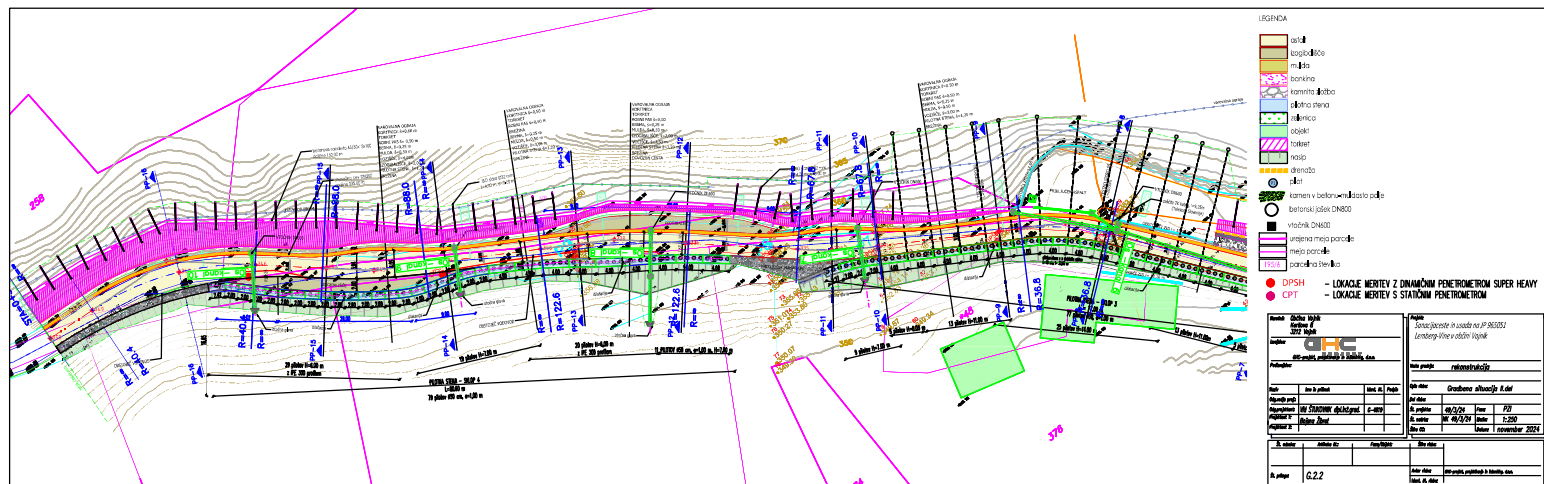
Slope stability ACCEPTABLE

G. GRAFIKE



Navedeni: Občina Vojnik Korčova 8 3212 Vojnik				Projekt: Sanacija ceste in uzada na JP 965051 Lemberg-Vine v občini Vojnik			
Izvajalec: GHC GHC-projekt, projektiranje in inženiring, d.o.o.				Predel gradnje: sanacija			
Podpisovanje:				Opis zbirni: Pregledna situacija			
Naziv: line in priključki		Ident. št.: 6-4619		Datum: 49/3/24		Faza: PZI	
Dag. udelež. projekt:		Vid. projektant: V. ŠTUKOVNIK dipl.inž.grad.		St. projekta: NK 49/3/24		Skala: 1:2000	
Projektant 1:		Bojana Žbrc		St. izdaje:		Datum: november 2024	
Projektant 2:				St. izdaje:			
St. oddaje:		Arhivski št.:		Faza/Objekt:		Stara risba:	
St. priključka:		G.1		Autor risbe:		GHC-projekt, projektiranje in inženiring, d.o.o.	
				Ident. št. risbe:			





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